



O PATRIMÓNIO CULTURAL CONSTRUÍDO FACE AO RISCO SÍSMICO

Seminário, 16 de Maio de 2013 Museu Nacional de Etnologia — Lisboa





NEW INTEGRATED KNOWLEDGE BASED APPROACHES TO THE PROTECTION OF CULTURAL HERITAGE FROM EARTHQUAKE INDUCED RISK









Development of integrated and knowledge based methodologies for the protection of Cultural Heritage assets from earthquakes on the basis of optimization and 'minimum intervention' approach.

Based on post-earthquake survey of damages after seismic events, drawbacks and limitations of the state-of-the-art technologies and approaches have been understood. Hence, the objective is to overcome the current shortcomings mainly related to:

- use of inadequate intervention techniques
- use of inadequate materials
- •use of inadequate tools for analysis or dated design methods
- analysis carried out on the basis of limited information



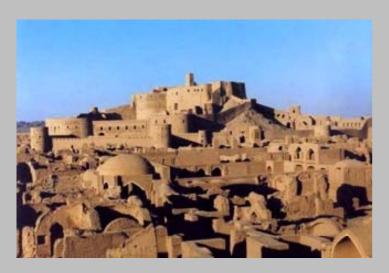


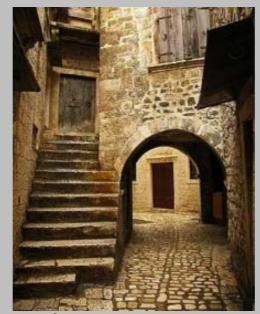














Studies and techniques for structural connections

Optimization approach for CH buildings

Testing and sub-structuring test methods

Monitoring and early warning systems

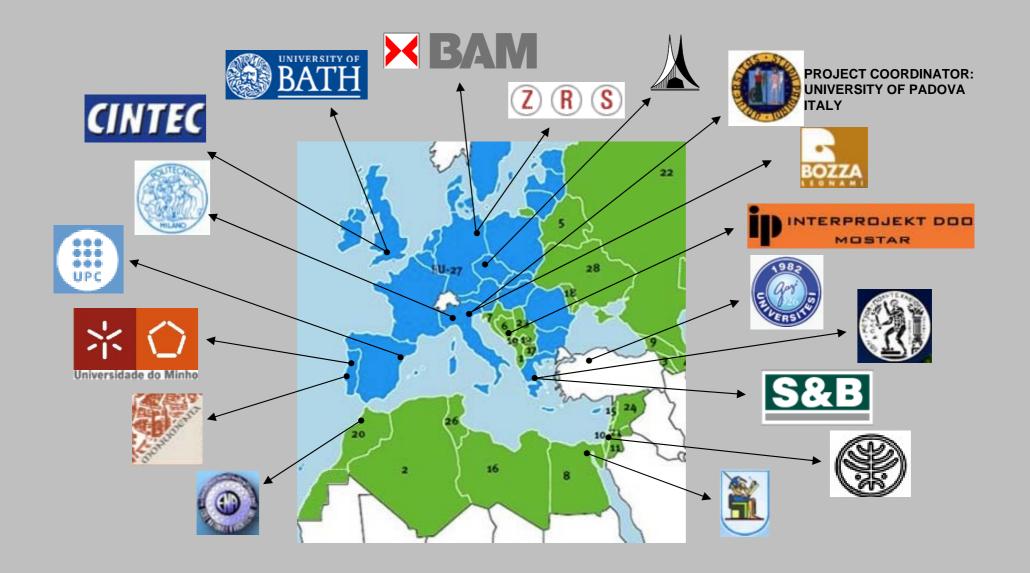
Integrated, multidisciplinary approach for CH

Standardization









- 18 partners
- 12 countries

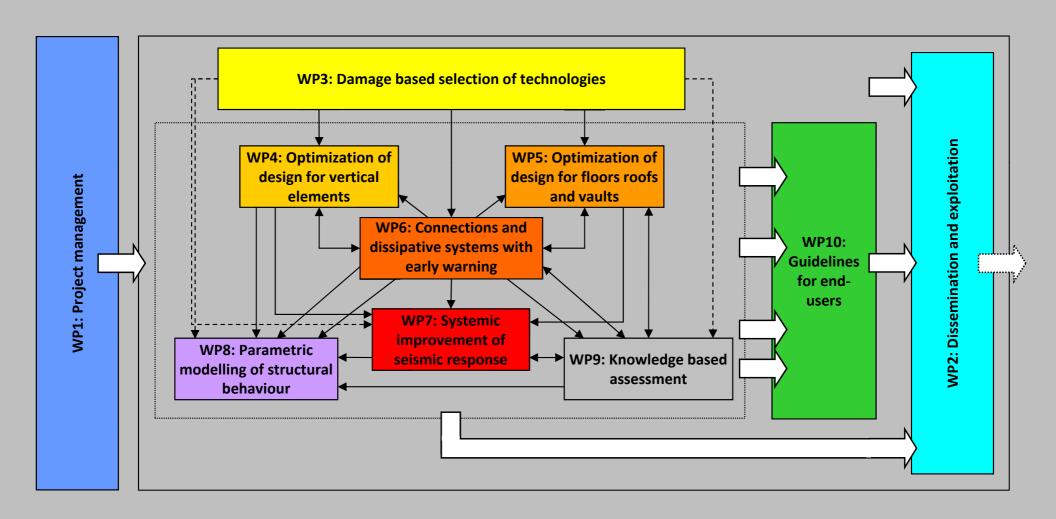
- 9 Universities
- 2 Research centres

- 6 Enterprises
- 1 Public body



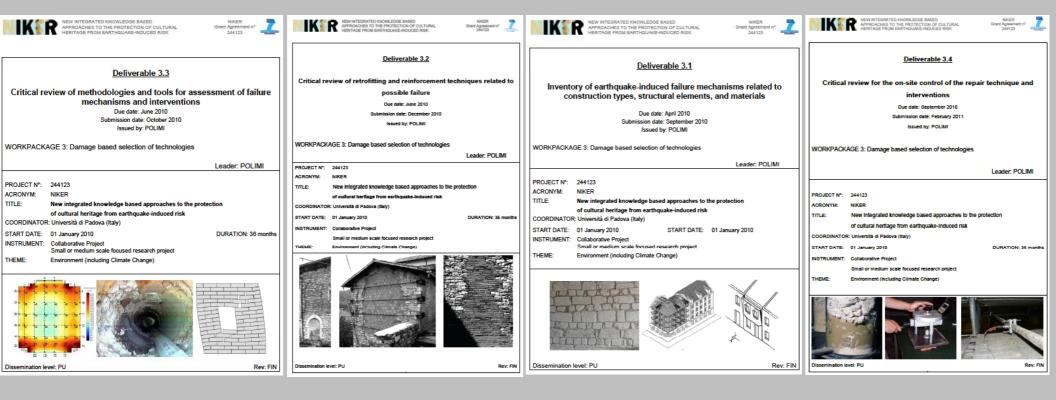


NEW INTEGRATED KNOWLEDGE BASED APPROACHES TO THE PROTECTION OF CULTURAL HERITAGE FROM EARTHQUAKE INDUCED RISK





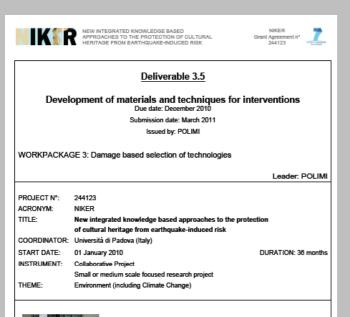




- **D3.1** Inventory of earthquake-induced failure mechanisms related to construction types, structural elements, and materials
- **D3.2** Critical review of retrofitting and reinforcement techniques related to possible failure mechanisms and requirements
- **D3.3** Critical review of methodologies and tools for assessment of failure mechanisms and interventions
- **D3.4** Critical review for the onsite control of the repair technique and interventions



D3.5 Development of materials and techniques for interventions



| TECHNIQUE: Local interven | | П | | Oleka | Linter | vention | | | _ |
|------------------------------|------------|--------------------------------------|-----------|----------|---------|------------|-------|----------|-----|
| | | | | | | | | | _ |
| | all 🗆 | pillar 🗆 | floor [| roof | | arch/vau | lt 🗆 | sub- | |
| element | | | | | | | | assembly | |
| Material | maso | nry 🗆 | stone | | bi | rick (| | adobe 🗆 | |
| | (from | | maso | nry | m | asonry | | | |
| | catalo | gue) | | | | | | | |
| | wood | | other | | D | escription | | | _ |
| Aim of the app | lication | / Advantage | s | | | | | | |
| | | | | | | | | | _ |
| Seismic mech | anism (t | from damage | catalogi | ie) | | | | | |
| | | | | -, | | | | | |
| Parameters to | estimat | te | | | | | | | |
| Property | | | | Range | of val | ios | Impro | vement | |
| Section monolit | hism | | | realinge | o. vai | | % | | _ |
| Tensile strength | | f. [N/mm²] | | | | | % | | |
| Compressive st | | f _c [N/mm ²] | | | | | 36 | | |
| Initial shear stre | | f _{v0} [N/mm ²] | | | | | % | | |
| Displacement o | | ψ [%] | | | | | 96 | | |
| Ductility | | u [-] | | | | | % | | |
| Energy diss. ca | pacity | Edia/Eino [%] | | | | | % | | |
| | , , | | | | | | % | | |
| Limits / Applic | ability / | Restrictions | | | | | | | |
| | | | | | | | | | |
| Documented s | eismic | performance | 8 | | | | | | |
| Documento : | , ciolinio | periormanoc | | | | | | | _ |
| Application pro | ncedure | e and remark | /G | | | | | | |
| Application pro | Joedule | o unu reman | 10 | | | | | | |
| Improved by the | na simul | Itananus usa | of | | | | | | - |
| improved by ti | ie alitiu | italieous use | UI. | | | | | | |
| Possible mista | kan in t | ha annliactic | | | | | | | _ |
| rossible mista | ikes in t | ne applicatio | TI . | | | | | | |
| | | | | | | | | | _ |
| Maintenance s | suggesti | ions and peri | odic cont | rois/moi | nitorin | g O | N SIT | E CONTR | SC. |
| | | | | | | | | _ 001111 | - |
| Long term per | | | | | | | | | |

Prelim
Param
Proper

Discomination level: PP

ONTROLS

Preliminary laboratory tests (on the material, on the assemblage, etc.)

References

| Parameters to estimat | te . | | | | | | | |
|----------------------------------|--------------------------------------|-----------------|--------|--|--|--|--|--|
| Property | | Range of values | Improv | | | | | |
| Section monolithism | | | % | | | | | |
| Tensile strength | f _t [N/mm ²] | | % | | | | | |
| Compressive strength | f _c [N/mm ²] | | % | | | | | |
| Initial shear strength | f _{v0} [N/mm ²] | | % | | | | | |
| Displacement capacity | Ψ [%] | | % | | | | | |
| Ductility | µ [-] | | % | | | | | |
| Energy diss. capacity | Eds/Einp [%] | | % | | | | | |
| | *** | *** | % | | | | | |
| Standards and/or Recommendations | | | | | | | | |

| | _ |
|--------------------------------|-------|
| In absence of addressed standa | irds: |

| procedure |
|-------------------|
| description |
| tools and |
| equipments |
| sample |
| dimensions and |
| characteristics |
| number of samples |

References

| pplication | | | |
|--|--|-----------------|---------------|
| roperty | | Range of values | Improvement |
| ection monolithism ensile strength compressive strength nitial shear strength itisplacement capacity juctility inergy diss. capacity | f₁ [N/mm²] f₂ [N/mm²] f₀ [N/mm²] f₀ [N/mm²] ψ [%] μ [-] Εω/Εμα [%] | | % % % % % % % |
| | | *** | % |
| tandards and/or Rec | commendations | | |

In absence of addressed standards:

application

After the

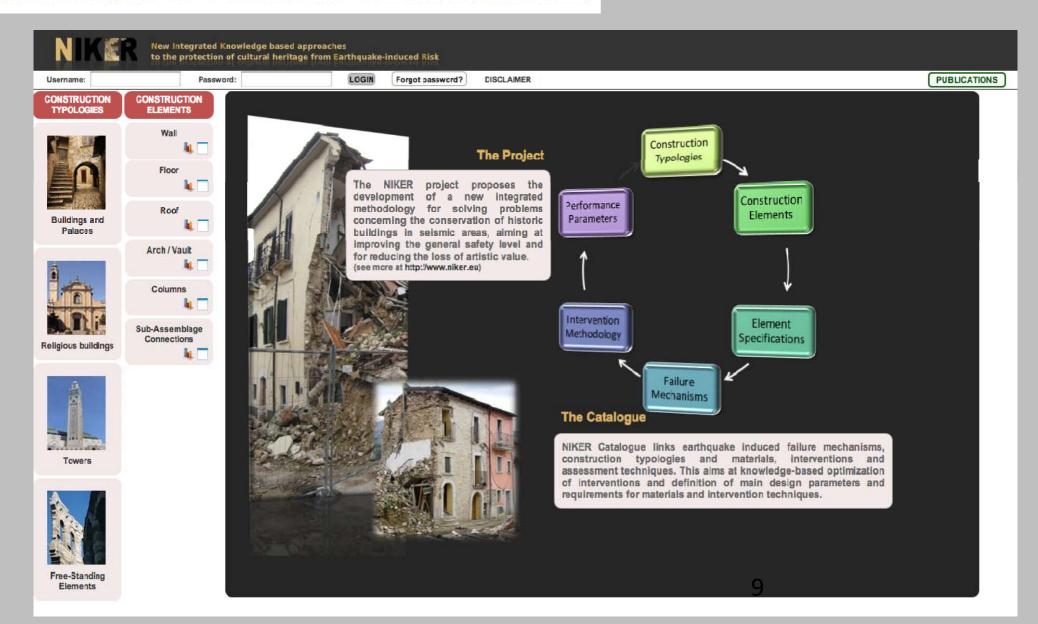
| | Procedures | |
|---|--------------------|--|
| | description and/or | |
| _ | complementary | |
| | tests | |
| _ | Tools and | |
| | equipments | |
| | investigation area | |
| _ | number of tests | |
| | References | |
| | | |





NIKER CATALOGUE

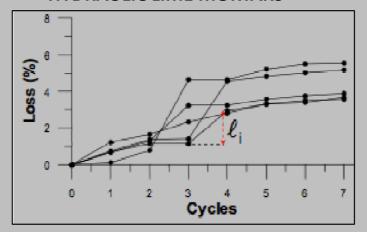
http://www.niker.eu





D3.7 – Critical evaluation of the effectiveness and compatibility of the new materials

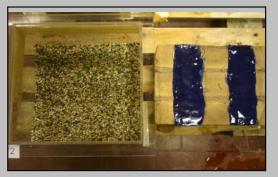
DURABILITY OF COMMERCIAL NATURAL HYDRAULIC LIME MORTARS



INJECTABILITY OF GROUT ADMIXTURES ON STONE MASONRY WALLS

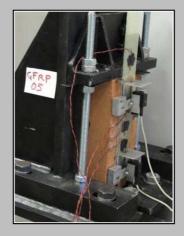


DURABILITY OF FRP APPLICATIONS
ON BRICK MASONRY

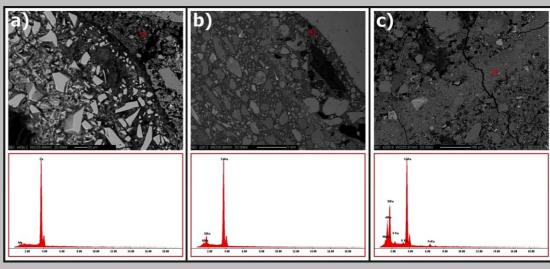




BOND BEHAVIOUR
OF FRP APPLIED ON
BRICK MASONRY



MICROSTRUCTURAL CHARACTERIZATION OF GROUT TO STONE MASONRY ORIGINAL MORTAR INTERFACE





WP4 - OPTIMIZATION OF DESIGN FOR VERTICAL ELEMENTS



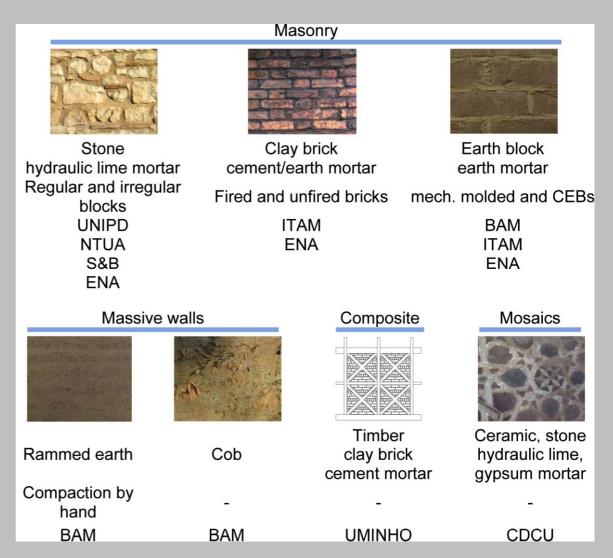
WP4 RATIONALE

Experimental campaigns carried out

Definition of:

 Adequate and feasible intervention methods for vertical structural elements

 Improvement of laboratory procedures for evaluating the intervention methods and specifications for laboratory specimens.





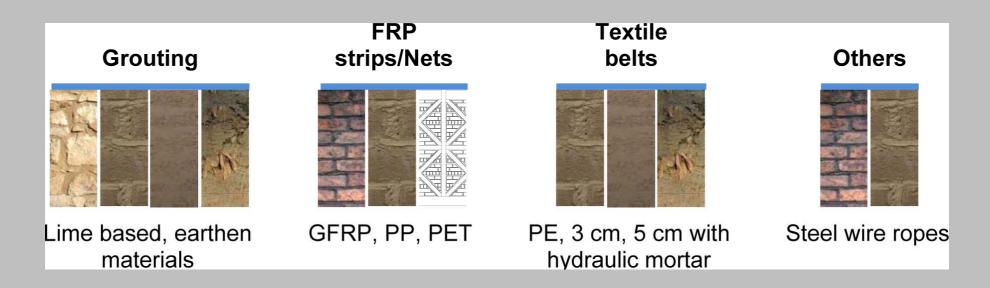
WP4 - OPTIMIZATION OF DESIGN FOR VERTICAL ELEMENTS



WP4 RATIONALE

Experimental campaigns carried out

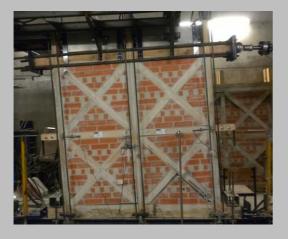
- Characterize the experimental behaviour of original and strengthened walls, in order to obtain information on the system performance and the main constitutive laws relevant for modelling.
- Numerical simulation of the experimental behaviour and perform parametric assessment to define critical mechanical parameters or define optimized design procedures.



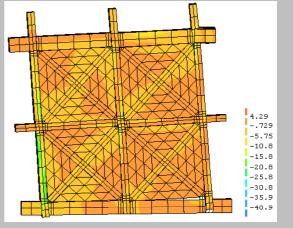


WP4 - OPTIMIZATION OF DESIGN FOR VERTICAL ELEMENTS

Intervention techniques, testing, modelling, analyses and derivation of design charts/equations.







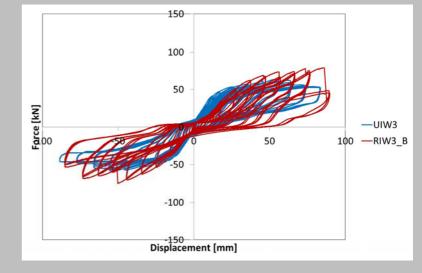














$$\begin{cases} f_{wc,0} = (V_{ex}/V) \cdot f_{ex,c} \\ f_{wc,s} = f_{wc,0} + (V_{inf}/V) \cdot f_{inf,s} \end{cases}$$



WP5 - OPTIMIZATION OF DESIGN FOR HORIZONTAL ELEMENTS

WP5 RATIONALE: WOODEN FLOORS

| FLOORS | | | | | | | | | |
|------------------------|----------------|--|--|---|----------------------|---------------------|---|----------------------|--|
| | | Testing Modeling | | | Parametric analysis | | | | |
| Level of investigation | Partner | Experimental tests | Analytical modeling | FEM Linear | FEM Non Linear | Analytical modeling | FEM Linear | FEM Non Linear | |
| Element | UNIPD BOZZA | Monotonic and cyclic tests on strengthened timber floors | Identification of in-plane stiffness and energy dissipation parameters | Calibration of global behaviour (in-plane strength and deformability) | | | | | |
| Element | ITAM | Experimental in-plane cyclic tests on authentic floor segments | Identification of in-plane stiffness and energy dissipation parameters | Calibration of global behaviour (in-plane strength and deformability) | | | Influence of porientation on stiffne | the floor | |
| Local | UNIPD BOZZA | | | Characterization and calibration of behaviour of connections | | | Influenc connections global behav floors | on the viour of | |
| | | | | | | | | | |



WP5 - OPTIMIZATION OF DESIGN FOR HORIZONTAL ELEMENTS



WP5 RATIONALE: VAULTS

| | VAULTS | | | | | | | | | |
|---------------|---------|--|--|-------------------------|--|---|--|--|--|--|
| Level of | | Testing Modeling | | | | Para | ametric an | alysis | | |
| investigation | Partner | Experimental tests | Analytical modeling | FEM Linear | FEM Non Linear | Analytical modeling | FEM Linear | FEM Non Linear | | |
| | UNIPD | Monotonic and cyclic tests on barrel vaults | moderning | Emed | | moderning | Effect | Enca | | |
| | UBATH | Pseudo- dynamic and cyclic tests on arches | | | | | | 4 | | |
| Element | UPC | | | | Modelling of strengthened vaults | | шини | Simulation of strengthening and failure modes | | |
| | UNIPD | Francisco (September 1994) | The state of the s | | | Calibration of design parameters for shear bond | S and a supplemental state of the state of t | The second secon | | |
| | GUNI | | | | | Interaction of parallel vaults with boundary conditions | \$22 VLT 303 | | | |
| | UMINHO | Shear bond of composites to brick units | Strain gauges | 3; 2; 1; Bonded area | Shear bond behaviour between bricks and composites | F FRPsheet Mortar | 5 13 22.5 Epoxy resin | 3D modelling of bond behaviour on prisms | | |
| Local | UNIPD | Bond of composites (pull-off, shear loads, dowel effect) to bricks | Analytical formulation of local mechanisms in strengthened conditions | | | Influence of local effects on load capacity. Calibration of pull-off bond | | | | |



WP5 - OPTIMIZATION OF DESIGN FOR HORIZONTAL ELEMENTS

WP5 RATIONALE: WOODEN ROOFS

| | ROOFS | | | | | | | |
|------------------------|----------------|---|--|--|----------------------|--|--|--|
| | | Testing | | Modeling | | Par | ametric analysi | |
| Level of investigation | Partner | Experimental tests | Analytical modeling | FEM Linear | FEM Non Linear | Analytical modeling | FEM Linear | FEM Non Linear |
| | UMINHO | Vertical loading on wooden trusses rescued from existing building and deterioration investigation on connections | | IX N | | | | |
| | ENA | Physical and mechanical characterization of wooden materials in timber elements | Verification of wooden floors and joists based on design criteria | A TOTAL OF THE PARTY OF THE PAR | | | | 765 |
| Element | UNIPD | | | Modelling of series of trusses | | | | Influence of corbel length on behaviour of serial trusses |
| | UMINHO | 200 15 10 45 60 75 90 101 Measurement | 120 135 150 165 180 185 210 | Modelling carrying performed in timber to | tests full-scale | Reliability assessment of timber trusses from NDT data | | |
| | POLIMI | | | Dynamic response of roof structures | | | Influence of geometric parameters in seismic vulnerability of timber trusses | 10 10 10 10 10 10 10 10 10 10 10 10 10 1 |
| Local | UNIPD BOZZA | M2 | asiz. | Calibration of mortise- tenon joint behaviour | | | | |



WP6 - CONNECTIONS AND DISSIPATIVE SYSTEMS WITH EARLY WARNING

DEPARTMENT ICEA
UNIVERSITY OF PADOVA

WP6 RATIONALE: TECHNIQUES FOR CONNECTIONS

- Testing procedures for the experimental validation of unreinforced and strengthened connections;
- Innovative techniques relying on ductility and energy dissipation;
- Indications on how to design connection strengthening and where to source parameters required in the process;
- Tackle the lack of information regarding:



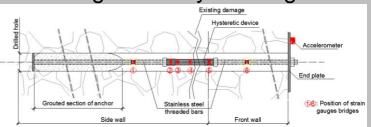
Less studied historic materials, such as earthen materials



Traditional reinforcement systems, such as timber lacing



Possible use of innovative systems for connection strengthening, monitoring and early warning







WP6 - CONNECTIONS AND DISSIPATIVE SYSTEMS WITH EARLY WARNING



WP6 RATIONALE: TECHNIQUES FOR CONNECTIONS

| Type of specimen | Specimen | Materials – Description of | Partner | Testing | | |
|---|----------|--|----------------------|-----------------------------------|---|--|
| | | the structure | | Type of tests | Strengthening | |
| Connection interface = 1 structural element + strengthening | | English-bond brickwork masonry | UBATH/ CINTEC | Monotonic pull-out | Metallic grouted anchors w/o <u>dissipative</u> <u>anchoring devices</u> | |
| | 201 | Earth block masonry/ rammed earth/ cob wall panels | BAM | Monotonic pull-out | GFRP/metallic grouted anchors | |
| | | Rubble stone masonry panels | UMINHO/ MONUMENTA | Monotonic pull-out | Grouted metallic anchors | |
| Whole connection = 2 structural elements + strengthening | | T-shaped double-bond brickwork masonry | UBATH/ CINTEC | Pseudo-static cyclic | Metallic grouted anchors w/o <u>dissipative</u> <u>anchoring devices</u> | |
| | | Timber carpentry joint | ITAM | Dynamic cyclic | Various (e.g. carbon plates, nails, <i>high-friction plates</i> , oak plates, pin) | |
| | | Rubble stone masonry panels and timber beams | UMINHO/ MONUMENTA | Monotonic pull-out | Metallic L profile bolted to beam and anchored to wall + <u>ductile anchor</u> | |
| Whole structure | | Three-leaf stone masonry walls with horizontal timber structures | NTUA | Recorded signals on shaking table | Timber-lacing | |



WP6 - CONNECTIONS AND DISSIPATIVE SYSTEMS WITH EARLY WARNING

PERFORMANCE PARAMETERS - E.G. ANCHORS

How should one dimension an anchor? What parameters does one need for the design? How are these parameters identified by tests? How do test compare with design codes and other references? How can be dissipative devices integrated in the design?



ULS:

$$F_{1U} = a_{U}M \le \frac{\pi d^{2}}{4} f_{y}n = F_{2U}$$

$$F_{2U} = \le \pi d_{2}lf_{b} = F_{3}$$

$$F_{2U} \le \sqrt{2}l(l + d_{2})\tau_{k} = F_{3}$$

DLS:

$$F_{1D} = a_D M \le F_{2D}$$

 F_{2D} : device activation load (yielding of hysteretic element/sliding of friction element



WP7 – SYSTEMIC IMPROVEMENT OF OVERALL SEISMIC BEHAVIOUR



Characterization of the seismic behaviour of original substructures and/or model buildings and the same strengthened with integrated interventions, coming from previous tests, by shaking table tests.



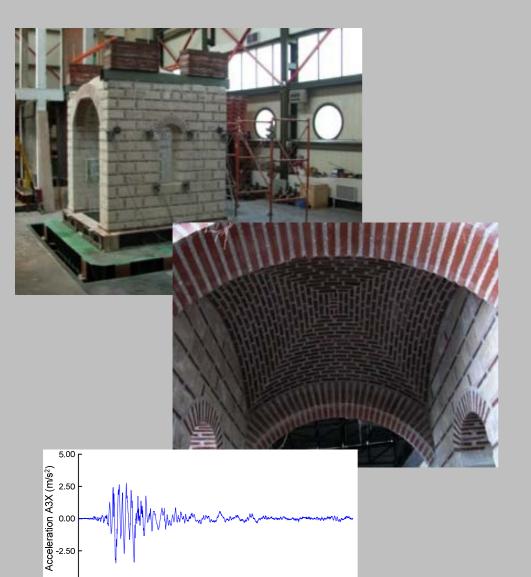


WP7 – SYSTEMIC IMPROVEMENT OF

OVERALL SEISMIC BEHAVIOUR



SHAKING TABLE TESTS OF SUB-STRUCTURES



15

Time (sec)

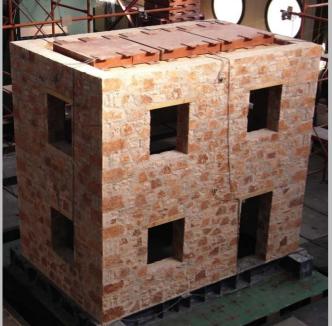
20

| 1 | Element | 94 | Three-leaf stone masonry |
|---|-------------|----|--|
| 2 | Element | | Adobe |
| 3 | Subassembly | | Adobe + light timber floor |
| 4 | Subassembly | | Adobe + heavy timber floor |
| 5 | Subassembly | | Adobe + light roof with stiff diaphragm |
| 6 | Subassembly | | Three-leaf stone masonry piers + timber floor |
| 7 | Subassembly | | Three-leaf stone masonry piers + brick arches and cross vault |



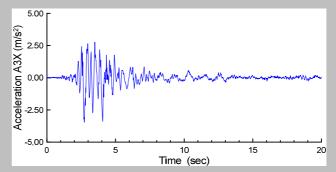
WP7 – SYSTEMIC IMPROVEMENT OF OVERALL SEISMIC BEHAVIOUR





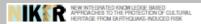
SHAKING TABLE TESTS OF MODEL BUILDINGS

| | Type of specimen | Specimen | Materials – Description of the | Partner | Type of tests | Testing Strengthening |
|---|------------------|----------|---|---------|---|--|
| 1 | Model building | | structure Three-leaf stone masonry + timber floors (double planking and steel ties) | UNIPD | Shaking table tests. Motion along two axes | (a) As-built (b) Grouting |
| 2 | Model building | | Three-leaf stone masonry + timber floors (double planking and steel ties) | UNIPD | Shaking table tests. Motion along two axes | (a) Grouting |
| 3 | Model building | | Three-leaf stone masonry + timber floors | NTUA | Shaking table tests. Motion along two axes | (a) As built (b) Grouting of masonry and enhancement of diaphragm action of floors |
| 4 | Model building | 1 7 | Three-leaf stone masonry + timber floors + timber laces | NTUA | Shaking table tests. Motion along two axes | (a) As built (b) Grouting (c) Enhancement of diaphragm action of top floor |





WP8 – SYSTEMIC IMPROVEMENT OF OVERALL SEISMIC BEHAVIOUR





Deliverable 8.1

Simplified and complex models of in- and out-of-plane response to be implemented in global analyses

> Due date: June 2011 Submission date: vvv Issued by: xxx

WORKPACKAGE 8: Parametric modelling of structural behaviour

PROJECT Nº: 244123 ACRONYM:

TITLE: New integrated knowledge based approaches to the protection

of cultural heritage from earthquake-induced risk

COORDINATOR: Università di Padova (Italy)

START DATE: 01 January 2010 **DURATION: 36 months**

INSTRUMENT: Collaborative Project

Small or medium scale focused research project THEME: Environment (including Climate Change)

Dissemination level: PU

D8.1 – Simplified and complex models of in- and out-of-plane response to be implemented in global analyses

D8.2 - Development of reliable numerical models assessment of connections and substructures





Deliverable 8.2

Development of reliable numerical models and assessment of connections and substructures

> Due date: December 2011 Submission date: XXX Issued by: UMINHO

WORKPACKAGE 8: Parametric modelling of structural behaviour Leader: UMINHO

PROJECT N*: 244123 ACRONYM:

TITLE: New Integrated knowledge based approaches to the protection

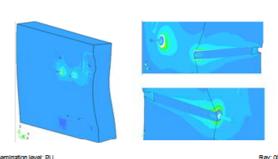
of cultural heritage from earthquake-induced risk

COORDINATOR/Università di Padova (Italy)

START DATE: 01 January 2010

INSTRUMENT: Collaborative Project

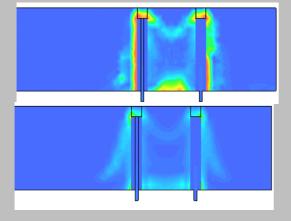
Small or medium scale focused research project Environment (Including Climate Change)

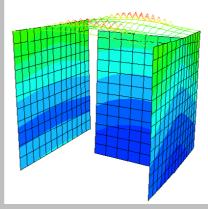


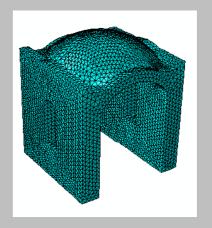
Dissemination level: PU



DURATION: 36 months

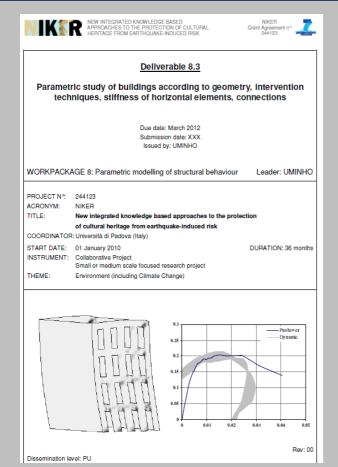




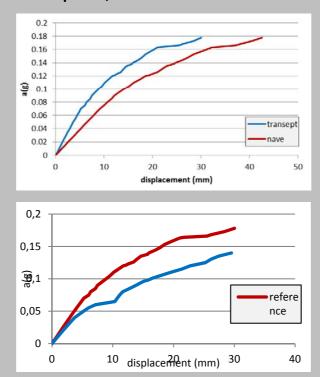


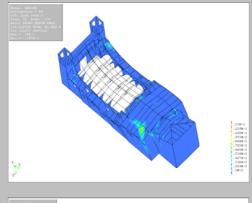


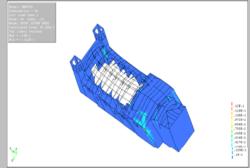
WP8 – SYSTEMIC IMPROVEMENT OF OVERALL SEISMIC BEHAVIOUR



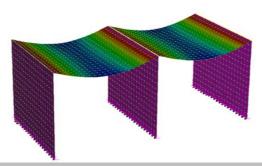
D8.3 – Parametric study of buildings according to intervention techniques, stiffness of horizontal elements, connections

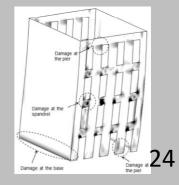






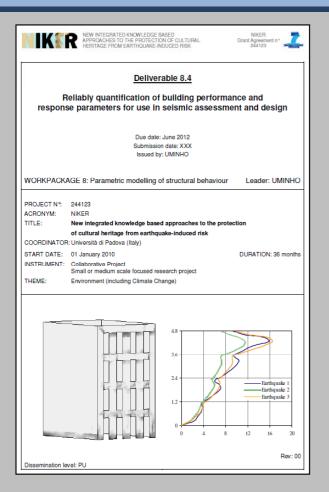
- Influence of floor stiffness on the distribution of horizontal loads among walls;
- Different types of constructions;
- Various geometric features;
- Various mechanical parameters;
- Various tyoes of analyses.



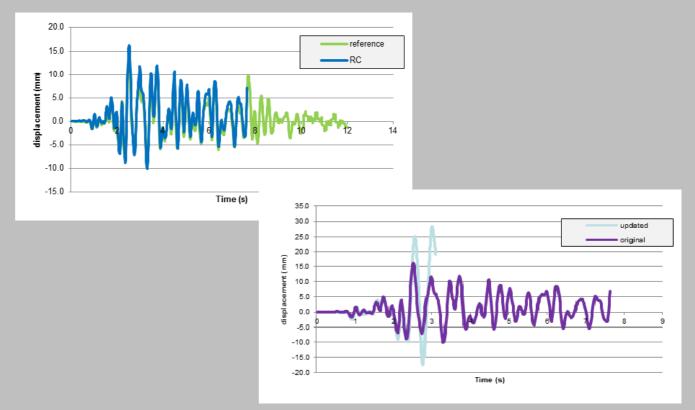




WP8 – SYSTEMIC IMPROVEMENT OF OVERALL SEISMIC BEHAVIOUR



D8.4 – Reliable quantification of building performance and response parameters for use in seismic assessment and design



- Displacement based design applied to a masonry structure;
- Seismic performance of strengthened two-story buildings;
- Seismic performance of strengthened historical single nave churches;
- Analytical approach for the seismic risk assessment of traditional earthen buildings



WP9 – KNOWLEDGE BASED ASSESSMENT



WP9 RATIONALE

Evaluate and validate proposed methods for knowledge based assessment

SELECTION (20 cases)

The collection of building has been chosen to cover different cases regarding:

- ☐ Significance of the building as CH
- ☐ Structural features and typology. Towers, fortresses, churches, palaces, other.
- ☐ Availability of information history (construction technologies, historical events...)
- □ Local seismicity. Low, moderate and high seismic locations
- ☐ Present and foreseen future uses and number of people at risk.
- ☐ In some cases, presence of valuable artistic contents.
- ☐ Present condition and damage. Almost intact to severely damaged/partially collapsed.
- ☐ Possibility of carrying out interventions.









WP9 – KNOWLEDGE BASED

ASSESSMENT





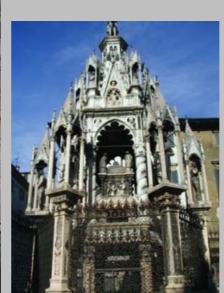












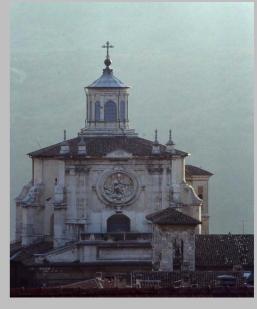
From left to right, top to bottom: Hagia Sophia Museum (Trabzon), Ras Cherratine Medersa (Fez), minaret of the Hadži-Alija's mosque (Počitelj, Bosnia-Herzegovina), Former Casa da Bragança, Foundation Head Office in Lisbon, Preceptory in Ambel, Spain, Os Jerónimos Monastery (Lisbon), Cansignorio Stone Tomb (Verona)



WP9 – KNOWLEDGE BASED ASSESSMENT













Case studies affected by the 2009 earthquake in l'Aquila:

- S. Biagio and S. Giuseppe churches, Spanish Fortress, S. Agostino church,
- S. Silvestro Church, Civic Tower, S. Marco church.



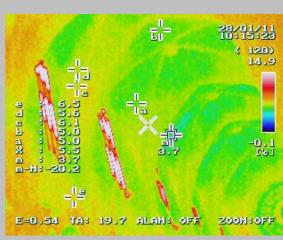
WP9 – KNOWLEDGE BASED

ASSESSMENT

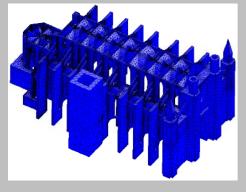


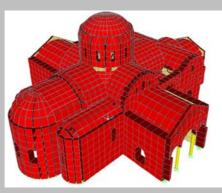
- ☐ Calibration of techniques to be applied on site
- **☐** Preparatory work
- ☐ Inspection
- Monitoring
- Numerical modelling
- Model updating
- ☐ Seismic assessment
- ☐ Intervention proposal
- **□** Definition of post-intervention programme
- **□** Conclusions



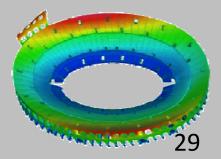














WP10 – GUIDELINES FOR END-USERS





- D10.1: GUIDELINES FOR DESIGN & EXECUTION OF INTERVENTIONS
- D10.2: GUIDELINES FOR ASSESS. & IMPR. OF CONNECTIONS & BUILDINGS
- D10.3: GUIDELINES FOR STICK-SLIP & HYSTERETIC DISSIPATIVE ANCHORS
- D10.4: GUIDELINES FOR SEISMIC ANALYSIS & KNOWLEDGE BASED ASSESS
- D10.5: INTEGRATED METHODOLOGY FOR PROTECTION & IMPROVEMENT OF CH

TECH

METH





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THANK YOU!

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